

THE EFFECTS OF TEMPERATURE ON THE MECHANICAL PROPERTIES AND MICROSTRUCTURE OF HIGH-STRENGTH CONCRETE CONTAINING POLYPROPYLENE FIBRES

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ABSTRACT

Concrete that possesses an extraordinary level of strength has discovered use in a wide variety of applications, some of which require it to withstand temperatures that are quite high. The building of bridges is an example of one of these applications. The considerable significance that polypropylene fibre plays in the spalling resistance of high strength concrete has been demonstrated by a large number of authors. These authors have demonstrated that this role plays a significant influence in the overall role. As a result of this effort, some vital information regarding the microstructure and mechanical characteristics of high-strength concrete has been acquired. These data will be used to improve the material. The concrete in question is polypropylene fibre-reinforced and can reach temperatures as high as 200 degrees Celsius during the heating process. When polyethylene fibre concrete with a high strength is heated to a maximum temperature of 170 degrees Celsius, the fibres readily melt and volatilize, which results in an increase in the concrete's porosity in addition to the production of microscopic channels. Investigations using differential scanning calorimetry (DSC) and thermogravimetric analysis (TG) shed light on the temperature ranges at which reactions of deterioration occurred in the high strength concrete. In the course of the examination using the SEM, it was discovered that the melting of fibres had resulted in the formation of additional pores and narrower channels inside the concrete. Melting the fibres resulted in the formation of these pores and channels in the material. During the course of the mechanical testing, it was observed that the splitting tensile strength, compressive strength, and modulus of elasticity of the material exhibited some very slight variations from one another. It's probable that the melting of the strands of polypropylene is what's creating these variations in the results.

KEYWORDS: Fibre-Reinforced, Polypropylene, Mechanical Properties

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INTRODUCTION

Because of its greater strength and stiffness, high-strength concrete offers a number of benefits, and the use of highstrength concrete has witnessed a growth in popularity over the course of the previous few years. One's level of selfassurance in using it will rise to the degree that they have a better understanding of its behaviours in a range of different contexts. The increasing use of high-strength concrete in construction projects is correlated with an increase in the risk of inadvertently exposing the material to temperatures over their design threshold. In order to accurately predict the reaction of structures that make use of high strength concrete both while they are being subjected to excessive temperatures and after they have been subjected to excessive temperatures, it is essential to have a solid understanding of the microstructural properties of high strength concrete that has been exposed to elevated temperatures.

When concrete is heated to high temperatures, its mechanical properties, also known as normal strength concrete, shift, which is something that has been a well-known phenomenon for several decades now. According to the findings of a number of high temperature exposure experiments that were carried out relatively recently, there are significant performance differences between high strength concrete and standard strength concrete when the concrete is subjected to extreme temperatures.

When concrete was subjected to rapid heating, Phan and Carino [9] presented a complete compilation of experimental findings on the material's changes to its mechanical characteristics as a result of this treatment. The results represent a synthesis of data from numerous individual experiments. When heated to temperatures of up to 300 degrees Celsius, normal concrete cracks more easily than silica fume concrete does, according to research that has been made public. In order to gain a better understanding of the phenomena known as concrete spalling, researchers looked into the thermal stresses and pore pressure in heated concrete members. It was hypothesised that a higher propensity to spall would be the result of unlucky combinations of qualities such as low porosity, poor permeability, poor thermal transfer, and a high moisture content in the material. This was a theory that was put up. Research has been done on a number of approaches to improve the high temperature resistance of high strength concrete in order to forestall the occurrence of challenges of this nature. This is being done in order to prevent the occurrence of difficulties of this nature.

Research was carried out by Sarvaranta and colleagues on the topic of the ways in which different types of fibres affect the fire resistance of mortar. According to the findings, the type of fibre has an effect not only on the heat and mass transfer, but also on the degree to which the mortar spalls when it is heated to higher temperatures.

Lie and Kodur investigated the thermal and mechanical properties of steel fibre reinforced concrete while the material was subjected to high temperatures. Lie and Kodur were able to determine the material's thermal and mechanical properties. They established that the effect of steel fibres on the mechanical properties of the tested concretes is substantially stronger than the effect of steel fibres on the thermal properties of the concretes. This was evidenced by the fact that the mechanical properties of the concretes were significantly improved. They arrived at the conclusion that the addition of steel fibres to concrete has the potential to boost the material's resistance to heat.

The findings of Hertz, who discovered that the inclusion of steel fibres in concrete did not make the material less vulnerable to spalling, are contradicted by the facts that are presented here. Because high strength concrete has a larger pore pressure than standard strength concrete, one of the concepts that has been thrown around over the course of the past seven years is to integrate artificial pores or channels into the matrix of high strength concrete. This would allow for more permeability throughout the material. This would make it possible to relieve the rising water vapour pressure to a level that is comparable to that of regular concrete that has an adequate number of capillary pores. Investigations exploring the feasible advantages of incorporating LMPF (low melting point fibre) into high-strength concrete have begun. Some of the authors carried out in-depth study on the impact that high temperatures have on the mechanical and microstructural characteristics of the material, and they documented their findings.

Researchers found that the higher the temperature, the higher the concrete's strength. When compared with the standard high-strength concrete, the high-strength fibre concretes exhibited a significantly reduced likelihood of spalling as a direct consequence of their temperature contact. This was found to be the case in both laboratory and field tests. They demonstrated that destructive spalling may be greatly minimised by including, in the concrete, minute quantities (on the order of 0.1% by volume), of fibres manufactured from a polymer with a low melting point. This allowed them to significantly reduce the likelihood of spalling occurring. These results, in general, are in agreement with those obtained by Hoff and Bilodeau et al. on the topic of the fire resistance of high strength concretes and fire resistance. Hoff and Bilodeau along with other others did research on this subject. According to the paper that was compiled by Breitenbuecker, the very first use of high-strength concrete that made use of polypropylene fibres took place in Frankfurt in 1995 et al. The 115-meter-tall "Japan centre" was built with high-strength concrete (HSC 105) that has a significant resistance to fire for many of the structure's structural sections. This allowed for the building to be erected. The amount of polypropylene fibres that were used was 2 kilogrammes per cubic metre.

Numerous scientific research have been conducted on the topic of the hygrothermal impact that a loss of coolant accident, abbreviated as LOCA, can have on a nuclear containment vessel. A rise in temperature from the air around them to a maximum of 160 degrees Celsius; a pressure of 650 kilopascals; and an increase in height to 650 metres were the conditions that contributed to the tragedy. Following this spike, there is a brief lull, followed by a period of gradual cooling that lasts for a good number of days.

Kuznetsov and Rudzinskii researched the high temperature heat and mass transport in a concrete layer that is used for the biological protection of nuclear reactors when they are subjected to critical heat loads. This layer is used in the process.

After an incident in which coolant was lost, Kontani and Shah [28] reported data describing the pore pressure and temperature distribution in concrete when it was subjected to a sustained high temperature of 171 degrees Celsius. This was done after the concrete was exposed to the high temperature for an extended period of time. These findings were based on the findings of an experiment. In the event that there is an accident, it is common knowledge that the temperature that is being held within the concrete containment tank may increase, but it cannot reach a temperature that is greater than 180 degrees Celsius. The rate at which concrete heats up is notably slower than the average for fire, when shown against the typical curve.

The primary objective of this study was to analyse the impact that an increase in temperature would have on the characteristics of two types of concrete that were designed for use in nuclear-related applications. One of these high-strength concretes had polypropylene fibers, while the other high-strength concrete did not contain any fibres at all. The heating curve that was used was not the standard fire curve; rather, it was a heating–cooling cycle that was somewhat comparable to RILEM's principles. The heating curve that was used was not the typical fire curve. The findings of this study add significant new information to what is already known about how high-strength fibre concrete behaves when subjected to temperatures that are outside of their regular operating range.

Test Program

The findings that were obtained when the test specimens were heated to a temperature of 200 degrees Celsius were contrasted with the results that were obtained when the specimens were maintained at a temperature of 20 degrees Celsius. After the test specimens had been heated to a temperature of 200 degrees Celsius, this procedure was carried

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out on them. In order to ensure that the findings of the test are as accurate as possible, the test specimens were heated for a specific amount of time, and during that time, any moisture that may have been present was allowed to completely evaporate. In order to carry out the experiment, concrete cylinders of two different sizes were placed into position. The dimensions of the cylinders were as follows: 110 millimetres by 220 millimetres and 160 millimetres by 320 millimetres. We conducted tests using two distinct combinations, each of which contained aggregate with a maximum size of twenty millimetres. Both of these different combinations yielded positive results. Both of these distinct arrangements contained instances of the aggregate.

We used a typical variety of Portland cement, which in its native language of French is known as CPA CEM I 52.5. This cement was manufactured by Lafarge. This particular variety of cement is also known by the designation CEM I 52.5. The type of superplasticizer known as sulfonated naphthalene formaldehyde condensate was selected to be used for this specific application since it was deemed to be the most suitable for the task at hand. In the course of the construction process, the silica fume served more as a supplement than as a replacement for the cement. This was due to the fact that silica fume is a gas. that there is a temperature. a temperature that there is some form of temperature. The incorporation of polypropylene fibres into the mixture led to a significant reduction in the quantity of water vapour pressure that was existing in the concrete prior to the raising of the temperature. This was the outcome of the concrete being exposed to higher temperatures. It was discovered that the optimum quantity of fibres to employ is somewhere in the neighbourhood of 1.5 kilogrammes per cubic metre.

The mathematical and computational model that Ahmed and Hurst devised was intended to predict the twodimensional thermal response of high strength concrete columns when high temperatures were introduced to the columns. Because the columns were subjected to high temperatures, the model was built as a response to such conditions. Because of this model, it is possible to make accurate predictions regarding the manner in which the columns will respond to temperature changes. The results of parametric studies have shed light on how important it is to conduct thermophysical material property testing when the materials in issue are being heated to high temperatures. This is something that was previously unknown. It has been demonstrated that these tests are necessary, and that is the primary reason why they are necessary.

THE RESULTS AS WELL AS THE DISCUSSION

Density

The dose of polypropylene fibres was determined to be 1.8 kilogrammes per cubic metre, and the length of the polypropylene fibres was 13 millimetres. The quantities of the two different concrete mixtures are listed in Table 1. After the aggregates, cement, and silica fume had been dry-mixed for two or three minutes (the fibres had been added in B3 concrete), the mixture was followed by the addition of water that had been mixed with superplasticizer. The mixing process was repeated for a further three minutes. All of the cylinders were cast in two layers using cardboard molds, and a vibrating table was used to condense the castings when they were completed. A piece of plastic sheet was used to cover each specimen. In order to ensure that the mass was properly cured, each specimen was sealed with a plastic cap and had a sheet of plastic placed within the cardboard mould. After that, the specimens were moved to the room where they would be moist-cured until it was time to conduct the tests. The specimens' densities were measured both before and after being subjected to the increased temperatures. Kilns that were powered by electricity were used as the source of heat for the equipment. The specimens were arranged in the kiln in such a way as to ensure that there was as little difference in temperature as possible between each specimen.

that were coupled to a data gathering unit were used to measure the temperatures both at the middle of the specimens and at their surface. After placing cylinders containing each combination in the kiln, the kiln was heated to the necessary temperature of 200 degrees Celsius at a rate of 0.5 degrees Celsius per minute. After being maintained at this temperature for three hours, the kiln was then turned off. In order to protect the specimens from being damaged by the sudden change in temperature caused by their removal, it was allowed to cool down first. There was no way to control the rate of the cooling. [30] The tests to assess compressive strength were carried out in accordance with the requirements outlined in the French standard NF P 18-406. The data were collected starting at the age of 62 days and going up to the age of 92 days. At a minimum of three specimens were examined and evaluated for every variable. DSC, TG, and SEM were utilised in order to perform an examination of the microstructure of both types of concrete.

MECHANICAL PROPERTIES

Initial Compressive Strength and Modulus of Elasticity

After the heating procedure, the specimens were allowed to come down to room temperature before the results of the tests were analysed. The properties of the specimens were tested not long before and after the heating process. In a short amount of time following the heating technique, the properties of the specimens were measured and analysed. Because of this, the researchers were able to figure out how the concrete mixtures they were studying reacted when exposed to greater temperatures. Utilizing the companion reference specimens as a means of computation helped make it possible to determine the starting strength of the heat-test specimens. This was accomplished by using the companion reference specimens. These companion reference specimens each comprised a number of cylinders and were heated to replicate various kinds of concrete. The specimens were arranged in pairs and served as a reference. The accompanying examples used these cylinders quite frequently as a point of reference throughout the text. These cylinders were used to conduct a strength test on the concrete in advance to heating it in order to guarantee that an accurate result would be obtained. This measurement was taken before the temperature of the concrete was raised. In order to prepare the reference test specimens for the heating tests, they were first broken up into smaller pieces. Because of this, the results were able to be more precise. The respective notations B1-20-C and B3-20-C relate to the inferences that can be drawn from these measurements. These inferences can be found in the table below. The modulus of elasticity of the B1 concrete was relatively equivalent to that of the B3 concrete, despite the fact that the two varieties of concrete had different compressive strengths. In spite of the fact that the B1 concrete had a lower compressive strength, this was nonetheless the case. In both instances, despite the fact that there were some key differences between the two, the overarching circumstances were the same. The influence that an increase in the cement concentration had on the material's modulus of elasticity was much less significant when contrasted with the effect that an increase in the cement concentration had on the material's compressive strength.

Residual Compressive Strength and Modulus of Elasticity

After being heated to 200 degrees Celsius, a number of different batches of concrete were created using the same cement, silica fume, and sand; however, the cement content in each batch was varied. These variations in mechanical properties were investigated. The performance of the specimen that was tested

When compared to the density of B1 concrete, which is also known as high-strength concrete without fibre, the initial density of B3 concrete, which is also known as high-strength polypropylene concrete, was lower. B3 concrete is also known as high-strength polypropylene concrete. It was discovered that concrete type B3, which had a

drop in density that was comparable to that which was displayed by concrete type B1, exhibited the same behaviour. The dryness of the cement paste was the primary factor in the change in the weight of the concrete. When everything was tallied up, the weight of the melted fibres did not amount to a number that could be considered significant in any way. According to the temperature readings that were recorded while the concrete was being heated, the amount of heat that was passed through the B3 concrete was less than the amount of heat that was transferred through the B1 concrete that was used as a reference. This was determined by comparing the two amounts of heat that were transmitted through the concrete. The quantity of heat that was transferred through each of the three different types of concrete was compared, which allowed us to come to this conclusion. It was determined that both the compressive strength and the modulus of elasticity had decreased following the initial heating to a temperature of 200 degrees Celsius. Both the material's non-heated strength and its non-heated modulus of elasticity experienced a reduction of between 28 and 37%, respectively. Additionally, the material's non-heated modulus of elasticity experienced a reduction of between 29 and 33%. here percentages are broken down by category, and here are the figures that represent them. According to the findings, the mechanical properties of the B3 concrete degraded at a pace that was faster than the rate at which the mechanical properties of the B1 concrete degraded across the whole temperature range that was tested. This was the case regardless of whether the concrete was placed at room temperature or at a temperature that was higher than room temperature. This was the case regardless of whether the concrete was placed at a temperature that was higher than room temperature or at the standard temperature of the room in which it was being placed. This was demonstrated by the fact that the rate at which the mechanical properties of the B1 concrete decayed was determined by comparing the two concretes. This result can be attributed to the investigation that was carried out on the two different types of concrete.

Initial and Residual Splitting Tensile Strength

Cylinders with dimensions of 110 and 220 millimetres were utilised as the test specimens for splitting. Table 4 presents the findings of the study. When polypropylene fibres were added to concrete, it seemed as though the heat resistance of the splitting tensile strength decreased. This is probable because the melting of the fibres created additional porosity and small channels in the mortar, which contributed to the observed phenomenon.

Differential Scanning Calorimetry

A piece of machinery known as the SETARAM Labsys 1200 was utilised in order to carry out the DSC analysis. This was done in order to achieve the desired results. Differential scanning calorimetry, which is often referred to as DSC, is a method of measurement that is utilised for the purpose of identifying the change in enthalpy in addition to the change in heat that a variety of various kinds of materials encounter. This may be accomplished by measuring the amount of heat that is transferred through the sample being scanned using the instrument. Scanning the sample with a differential heating element is one way to achieve this goal. The container containing the mortar paste has a portion of it removed so that it can be used to form a sample, which is then subjected to a technique for controlling the temperature. During the length of its operation, this programme keeps the temperatures at a steady level, in addition to keeping the heating rates at a constant level. The measurements of heat flow to and from the sample can be used to derive the temperature ranges that correspond to the transitions. These measurements were taken in both directions, to and from the sample.

When a cementitious material such as concrete is heated to temperatures between 100 and 900 degrees Celsius, a number of chemical and physical events take place. These temperatures span from the low 100s to the high 900s. Both high strength concretes were heated, and the reactions that occurred as a result of the heating were analysed. The pace of heating was 10 degrees Celsius per minute. Figure 2 displays some DSC curves for you.

Between temperatures of 100 and 250 degrees Celsius, a number of distinct physical processes took place, including the evaporation of water from the cementitious matrix at 110 degrees Celsius, the dehydration of CSH, fibre shrinkage, and melting at 170 degrees Celsius. These findings are extremely in line with those that were reported by Phan and Carino [9]. It is possible to observe two endothermic peaks in the same temperature zones, which range from 100 to 250 degrees Celsius. The following transformations can be found in the DSC curves of both the B1 and B3 concretes: the process of water evaporation at a temperature of 110 degrees Celsius, the melting of fibre at a temperature of approximately 170 degrees Celsius, and the beginning of the CSH dehydration process at a temperature of 170 degrees Celsius

There is a peak in each of the curves that corresponds to a temperature of approximately 480 degrees Celsius. It's likely that the existence of some portlandite is to fault for this phenomena. Even though high-strength concretes B1 and B3 contained silica fume, which is known to react with calcium hydroxide during the hydration process of cement, it is likely that a non-negligible amount of portlandite was present. This is because silica fume is known to react with calcium hydroxide during the hydration process of cement, it is common knowledge that silica fume will react with calcium hydroxide. This is the reason why. In an earlier piece of work, Weigler and Fisher [3] made the observation that there was a disparity between the two outcomes, and they referred to it as a "discrepancy." At this time, there is no explanation that can be presented that can even remotely be thought of as adequate.

When heated to 573 degrees Celsius, the crystal structure of quartz shifts from a rhomboedric shape to a hexagonal shape. This occurs because the rhomboedric structure is more stable at lower temperatures. Temperatures of approximately 870 degrees Celsius have the potential to show a major peak. The primary contributor to the occurrence of these phenomena is the dissolution of calcium carbonate and CSH phases, which can be thought of as the "key factor."

When tested at a temperature of 250 degrees Celsius, it was discovered that the behaviour of B3 concrete was startlingly comparable to that of B1 concrete. When put head to head with one another, these two varieties of concrete reveal this to be the case. This is shown to be the case when these two types of concrete are contrasted with one another in a head-to-head competition. After bringing the temperature of the concrete up to a higher level, we came to the conclusion that this was the case. Between the temperatures of one hundred and two hundred and fifty degrees Celsius, the effect of the fibre is at its peak strength. This happens right before the fibre enters a stage in which it is completely molten across its entirety.

Theremogravimetry

The SETARAM Labsys 1200 was the instrument that was utilised in order to carry out the thermogravimetry analysis. A sensitive balance is required in order to monitor the change in weight of the material sample as a direct result of the variation in temperature. The rate of heating in air was 10 degrees Celsius per minute. The findings are presented in Table 5. Specimens of B1 and B3 concrete were heated to 200 degrees Celsius and left unheated before having samples cut from them.

Initial moisture levels in B1 concrete were not significantly different from those in B3 concrete. while the TG and DSC curves were analyzed, it was discovered that the temperature ranges that led to the weight loss that was reported in the TG curves were the same temperature ranges that were detected for breakdown processes. This was discovered while comparing the two sets of curves. These temperatures were measured, and the results suggested that there had been a reduction in weight. When it came to the reduction in weight that was caused by heating, the behaviour of the B1 and B3 concretes was extremely similar to one another. This was the case when compared to each other. This was the case across the board with regard to the experiment. In this respect, the two concretes were comparable to one another when compared side by side. The result that the C– S– H dehydration in non-heated concrete was much higher between 100 and 450 °C in comparison to that in heated concrete should not come as a surprise to anyone; this is something that should be anticipated. According to the results of the decomposition of Ca(OH)2 that took place at temperatures ranging from 450 to 520 degrees Celsius, it would appear that both types of concrete contained the same amount of portlandite. The temperatures at which the reaction took place were compared, and this enabled us to come to this conclusion. The fact that these tests were carried out across such a broad temperature spectrum led to the discovery of this result.

Scanning Electronic Microscopy

In the course of carrying out research on the topic, a scanning electron microscope was utilised in order to analyse samples in both their hot and cold states. This was done in order to better understand the differences between the two conditions. As shown in Figure 3, high-strength concrete that has not yet been burned has polypropylene fibres that have been distributed evenly throughout the material. The figure demonstrates this point quite clearly. The vestiges of the fibres can be seen in figure 4, which illustrates the state in which they were discovered prior to the melting process. The melting process has now been completed, and the remnants of the fibres can be seen. When heated to 200 degrees Celsius, the ability of polypropylene fibres to self-assemble into a three-dimensional structure is completely eradicated. Both the B1 and B3 concretes exhibited noticeably different levels of porosity after going through a heating process at a temperature that reached 200 degrees Celsius. This was due to the fact that the B1 and B3 concretes were heated to the same temperature. This divergence emerged as a result of the fact that B1 concrete possessed a higher permeability than did B3 concrete. The permeability of B3 concrete was significantly lower. When heated to a temperature of 200 degrees Celsius, the high strength polypropylene fibre concrete will cause the fibres to begin to rapidly melt and volatilize. This will cause the concrete to lose its strength. This results in the production of new holes and small channels inside the concrete, which may serve the function of alleviating excessive moisture pressures that are present within the concrete. Moreover, the formation of these new holes and small channels is caused by the fact that the concrete is exposed to water. Because of the concrete's prolonged contact with water, the formation of these voids and channels has taken place. The research that was carried out with the assistance of the SEM revealed, as a consequence, that there was evidence of melted fibres (Fig. 5). Because of the relaxing that took place as a direct result of the heating process, the length of the polypropylene fibres was reduced. This was accomplished by shortening the fibres. This transpired as a result of the heating of the fibres. This result was partially attributable to the manner in which the heat was applied. They succumbed in the end to the heat and melted after being subjected to even more intense heating than they had previously experienced. When high strength concrete is heated, the incorporation of fibre into the mixture unmistakably has an impact on the porosity of the material. This is not something that can be questioned or discussed in any manner, shape, or form at all. Because

of this, there is a possibility that the pore pressure within the high-strength concrete will fall as a direct result of this. This is due to the fact that this is a direct consequence of this. In addition to that, here is a possible course of events. The amount of moisture that is expelled from the material is indirectly related to the level of porosity that the material possesses, which in turn is determined by the fibres. In turn, the quantity of moisture that is released from the material has an effect on how much of this effect is felt. Because of this, the likelihood that this will occur is significantly raised as a direct consequence of this.

DISCUSSIONS

Both at room temperature and when the temperature rises above it, the microstructure determines how much water is expelled from the concrete. As a result, the pore structure of the high strength polypropylene fibre concrete may have a significant impact on the spalling behaviour of the material when it is exposed to high temperatures.

It is possible that the melting of polypropylene fibres will have a positive effect on the behaviour of fibre high strength concrete when it is exposed to high temperatures. In the event that the high strength concrete is subjected to intense high temperatures, not all of the water is ejected from the material quickly enough. As a direct consequence of this, evaporation will take place at greater temperatures, in addition to the production of high pressures within the paste [10–12]. The additional porosity and narrow channels that are formed as a consequence of the melting of polypropylene fibre may be able to lessen the chance of spalling in the concrete by lowering the internal vapour pressures that are present in the concrete. This is accomplished by reducing the amount of internal water vapour pressure that is present in the concrete. The process of melting makes it possible to attain this goal. The microstructural behaviour of the material can, of course, be influenced by a number of factors, including the size of the fiber, the amount of the fibre that is present in the material, and both of these factors together.

CONCLUSIONS

This study was carried out with the purpose of collecting data regarding the effect that temperatures of up to 200 degrees Celsius had on the properties of two distinct varieties of concrete that were planned for use in nuclear-related applications. The research was carried out with the objective of getting this information as soon as possible. The purpose of the study was to ascertain whether or not the concrete would be able to endure the temperatures by testing it. The investigation that was carried out with the goal of acquiring this information also included gathering it as one of its objectives. The researchers looked at a high-strength concrete that contained polypropylene fibres as well as a high-strength concrete that did not contain any fibres in its composition. Both types of high-strength concrete were designed to be extremely durable. It was found that the two different types of high-strength concrete have strengths that are equivalent to one another and were therefore interchangeable. During the course of this particular piece of research, an analysis of the mechanical properties of the concrete was performed both after it had been allowed to cool to room temperature and after it had been heated to a temperature of 200 degrees Celsius. Both of these analyses were carried out in order to compare and contrast the results. Because the polypropylene fibres will melt during the process of heating the material, it is likely that the material's residual compressive strength, modulus of elasticity, and splitting tensile strength will be slightly altered as a result of the addition of polypropylene fibres (1.8 kg/m3). This is because the polypropylene fibres will melt during the process of heating the material. This occurs as a result of the fact that the polypropylene fibres in the material will melt as a result of the heating procedure. This takes place

as a consequence of the fact that the polypropylene fibres will melt during the process of heating. It appeared as though the heat resistance of the concrete, along with its other mechanical properties, had deteriorated as a direct result of the addition of polypropylene fibres to the mixture of components used to manufacture the concrete. This was the case because it appeared as though the polypropylene fibres had an adverse effect on the heat resistance of the concrete.

Transmission grating, differential scanning calorimetry, and scanning electron microscopy were used in order to do an analysis of the microstructure of both kinds of concrete that were put through their paces in this study. The goal of this inquiry was to determine the similarities and differences between the microstructures of the two types of concrete. This was done so that an inquiry could be conducted into the microstructure of both varieties of concrete that were utilised in the course of this research and put through their paces. Both types of concrete were subjected to an amount of stress that was equivalent in magnitude. The findings of the thermogravimetry and differential scanning calorimetry tests showed that there was not much of a difference between the two different types of concrete that were evaluated for this study. These findings were disclosed by the findings of the tests. Buildings were crafted with both types of concrete at various stages of the process. Both types of concrete were utilised in the construction of the many bridges that were built. In numerous construction projects carried out all across the world, wet and dry variations of the material known as concrete were applied. There is not the faintest shred of doubt in anyone's mind that the temperature ranges in which the various phases of decomposition took place were, in all essential respects, interchangeable with one another. This is due to the fact that there is not even the tiniest bit of uncertainty in anyone's mind. In the course of the investigation, scanning electron microscopy discovered irrefutable evidence that the fibres had melted, which led to the development of a greater number of pores in the material. This was the smoking gun that the investigators had been looking for. This finding is indisputable and cannot be contested. When compared to the high-strength concrete that was manufactured with polypropylene fibres and then subjected to heating at a temperature of 200 degrees Celsius, the high-strength concrete that served as a point of comparison exhibited a porosity after the heating process that was noticeably distinct from the highstrength concrete that was the subject of the study. The high-strength concrete that served as a point of comparison was heated to the same degree as the other concrete. Both of them might be distinguished from one another using a variety of various criteria and characteristics. Because of this, it is highly possible that the vapour pressure in the high-strength concrete that was built using polypropylene fibres would fall when it was first heated up. This is because of the fact that the vapour pressure is directly proportional to the temperature. This is because, in compared to other types of fibers, polypropylene fibres have a higher porosity level. Other types of fibres also have a higher density. This is due to the fact that polypropylene is a substance that demonstrates a level of resistance to water that is regarded as being adequate. This takes place as a consequence of the action of polypropylene fibres in response to the application of heat to the material. The reason for why this is the case may be discovered in the line that came before it, which also contains the explanation. Because of this property, there is a lessened probability that the concrete may splinter into potentially hazardous bits in the event of a collision. This is because of the fact that the concrete is dense. This is as a result of the increased density of the concrete.

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